Paul J. Petretti

Civil Engineer & Land Surveyor
Certified Wetland Delineator
Certified Floodplain Manager

CIVIL ENGINEERING - LAND SURVEYING & MAPPING - SITE DESIGN & PLANNING ENVIRONMENTAL & GEOTECHNICAL - DRAINAGE & STORMWATER QUALITY EROSION & SEDIMENT CONTROL STORMWATER POLLUTION PREVENTION PLANS FLOODPLAIN MANAGEMENT & HYDROLOGY

November 3, 2016

Village of Hastings-on-Hudson 7 Maple Avenue Hastings-on-Hudson, New York 10707

Attention: Kathleen Sullivan, Chairperson and Planning Board

Re: Subdivision Application of Dean L. Wetherell and Marie T. de Bethune

Wetherell, 196 Warburton Avenue, Hastings-on-Hudson, NY 10706.

Site drainage - October 30 Soil Test Pit

Dear Chairperson and Planning Board:

We appeared before the Planning Board for a pre-application discussion of the resubdivision of the above referenced premises. This proposal being a two lot subdivision that will create one lot for the existing residence and a new building lot; this application will require an action before the Zoning Board of Appeals for area variances.

The new lot on the east side of Warburton Avenue has a depression behind a low stone wall along line of Warburton Avenue. In the depression is a drainline that catches runoff from the steeply sloping lands that rise to the back of the lot to the right-of-way of the Old Croton Aqueduct (OCA). Along the length of the OCA we find stone culverts that catch and convey runoff under the OCA; at times significant stormwater flows have been observed and we have a video taken at the time of a heavy rainfall event to confirm this.

The drainline we find in the depression catches all the runoff from the lot and the OCA. Looking into the line we see a very clean corrugated pipe that appears to make a change to a reinforced concrete pipe at the location of the low property wall line of the lot. We assume this drainline is connected to a deep drain manhole on the west side of Warburton Avenue with an outlet at the base of a very high stone retaining wall; the base of the wall being at what was once a ravine before Warburton Road was set out.

Page 2/3 Subdivision Application of Dean L. Wetherell and Marie T. de Bethune Wetherell, 196 Warburton Avenue, Hastings-on-Hudson, NY 10706.

Site drainage - October 30 Soil Test Pit

A test was made by inserting a hose in the drainline in the depression allowing water to run constantly. After a time we see water running in the open channel at the base of the high retaining wall on the west side if Warburton Avenue. This confirms that the pipe we find in the depression is connected to a working drainage system.

On October 30 Th. a test pit was made on the lot in the depressed area with a small backhoe to a depth of 6.8 feet and an in-place test to determine how well the soil will drain was made by putting a hose into the test pit. Water filled the test pit to a depth of approximately thirteen inches (13") and remained at that level as the hose continued to run constantly and the test pit did not fill and kept taking water.

The soil excavated from the test pit is a clean granular fill with a few bricks and rock with no signs of deleterious matter and not very dense.

We determined the rate to the test pit by filling a bucket with the hose to be a flow rate of 6.1 Gallons per minute. The soil is accepting water at a very high rate and using the dimensions of the test pit we calculate a soil adsorption rate (SAR) of 72.5 cf/sf/day (Cubic Feet per Square Foot Per day), which for soils in this area is a very high SAR. Soils rates for the glacially loaded soils in the Rivertowns range between a low of 1.0 to a high of 13.0 cf/sf/day.

The water that discharges from the stone culvert under the OCA at the back of the lot runs down the slope and although this flow rate is witnessed to be significant there is no visible signs of erosion of the steep slope. The existing drain ine in the depression would be removed as part of the action of building a house and driveway. New drainage works would be installed, a circular stone catchments about one-third the way up the slope to catch the runoff from the OCA, a new large diameter drianline to move water from the catchment around the newly constructed house with a few drain manholes to make the necessary turns in the drainline around the house to be connected to the drainline we see at the wall along the east side of Warburton Avenue.

A drywell can be installed to accommodate the increased impervious area; however, at times when you have such a high soil rate it may be best to make a direct connection to the existing drainline.

The question raised by several board members is can we deal with the drainage aspects related to development of this new lot. Drainage issues can be addressed by the above means.

Page 3/3 Subdivision Application of Dean L. Wetherell and Marie T. de Bethune Wetherell, 196 Warburton Avenue, Hastings-on-Hudson, NY 10706.

Site drainage - October 30 Soil Test Pit

We attach several photos taken at the time the test pit was made and a marked-up plan showing the existing and proposed drainage features. Should you have any further questions on this matter, please do not hesitate to call.

Very truly yours,

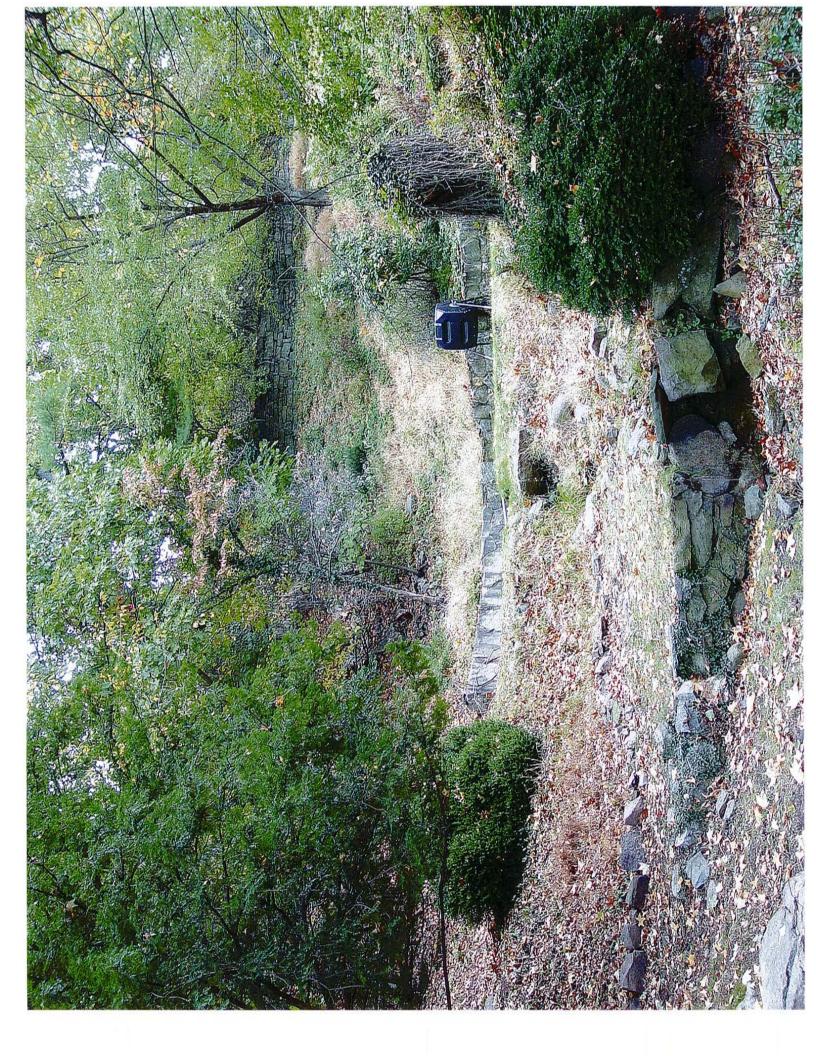
Paul J. Petretti, P.E., L.S., CFM

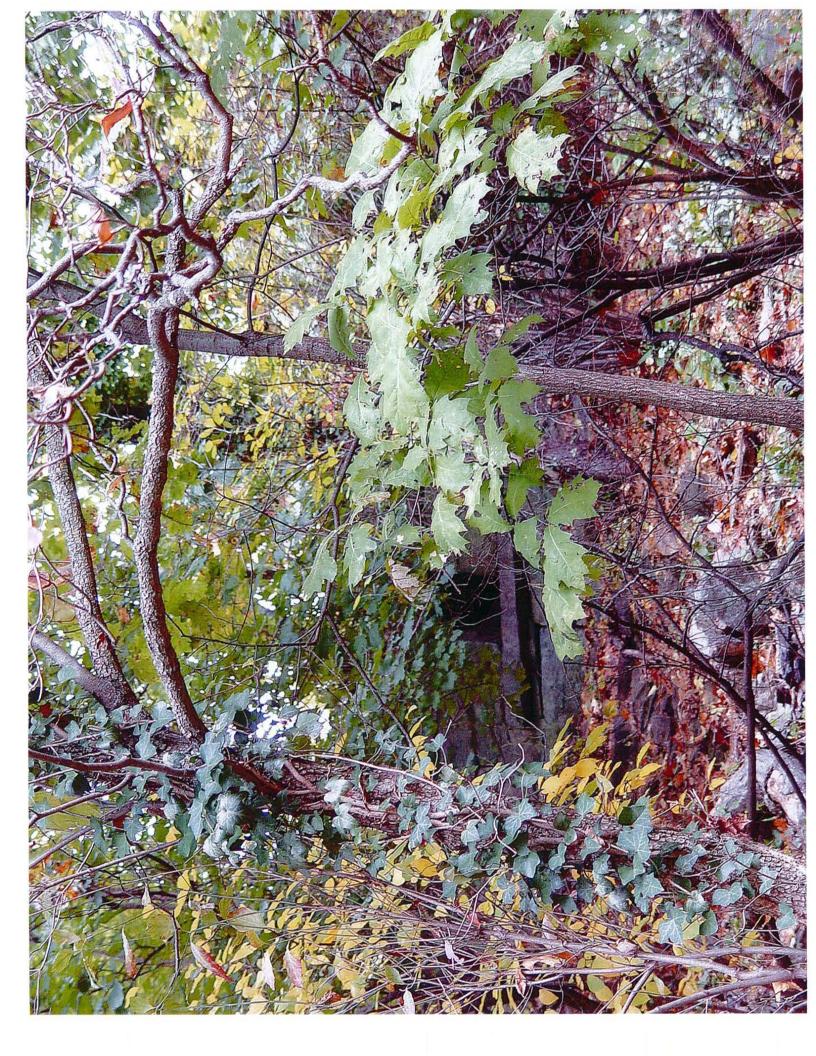
cc: C:\My Documents\VOH Projects|196 Warburton Avenue\ 196 Warburton Avenue Correspondence.doc

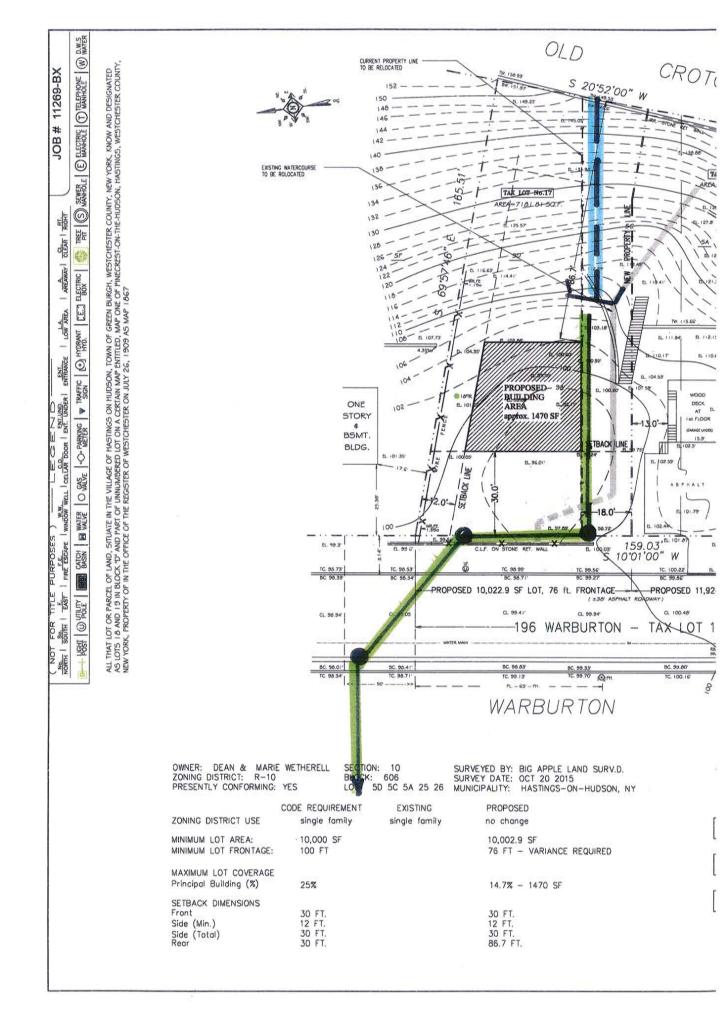
Applicant Dean L. Wetherell and Marie T. de Bethune Project architect, Mr. Tomasz Lopinski











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CIVIL ENGINEERING - LAND SURVEYING & MAPPING - SITE DESIGN & PLANNING ENVIRONMENTAL & GEOTECHNICAL - DRAINAGE & STORMWATER QUALITY EROSION & SEDIMENT CONTROL STORMWATER POLLUTION PREVENTION PLANS FLOODPLAIN MANAGEMENT & HYDROLOGY

GLOBAL POSITIONING SYSTEM AND LASER SCANNING SERVICES FOR THE SURVEYING AND MAPPING INDUSTRY

DRAINAGE REPORT
PALISADES VIEW SUBDIVISION
196 WARBURTON ROAD
HASTING-ON-HUDSON, NEW YORK

May 5, 2017



INTRODUCTION

A drainage letter report dated November 3, 2016 was submitted prior to making a formal application for the two lot subdivision "PALISADES VIEW SUBDIVISON".

Please refer to copy of the report in Appendix A of this report.

This drainage report will address the stormwater runoff that passes through the Lot 1 of the proposed subdivision, the area tributary to the culvert that discharges to the lot.

There are three (3) sub-area tributary to the drainage way that runs through the proposed lot, a long swale that runs parallel with the trail-way on the Old Croton Aqueduct that collects some runoff from the trail-way, a second sub-area being the land with single-family houses east of the OCA trail-way that also front on Pinecrest Drive and a portion of Pinecrest Drive itself.

These tree (3) sub-areas form a combined area tributary to the waterway on Lot 1 of the proposed subdivision, all with different times concentration. It appears from observation during recent rainfalls that 1.5 inches of rainfall in a twenty-four hour period will show as a continuous flow out of the box type culvert that will carry runoff from the swale on the east side of the OCA to the box culvert; please note we have yet to find the inlet to this box culvert.

METHODOLOGY

A traditional analysis using techniques described in the SCS publication "URBAN HYDROLOGY FOR SMALL WATERSHEDS Technical Release No. 55" can be used to determine the rates of runoff and storm water storage requirements.

Times of concentration calculated by Lag TC method

The three sub-areas are:

- 1. Pinecrest Drive, Area = 0.23 Acres, Tc = 7 minutes
- 2. Lands and houses east of the OCA, Area = 0.61 Acres, Tc = 2.0 minutes
- 3. OCA swale, Area = 0.46 Acres, Tc = 2 minutes

Rational Method and TR=55 hydrographs

Combine the three and we see that 7.0 CFS could flow through the site

Rational method calculates 8.6 CFS.

The flow from the waterway will be brought around the proposed house on Lot 1 by way of a catchment into a fifteen-inch (15") drianline, with a capacity of 12.2 cfs which is greater than the anticipated flow to the lot.

APPENDIX A

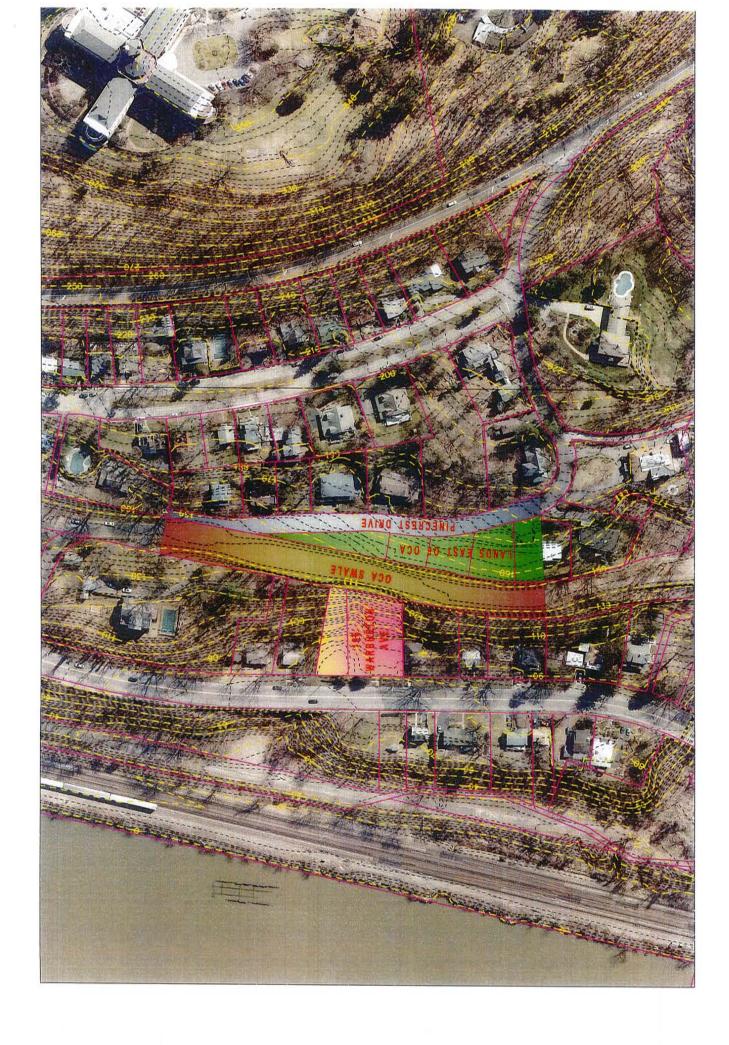
November 3, 2016 letter report

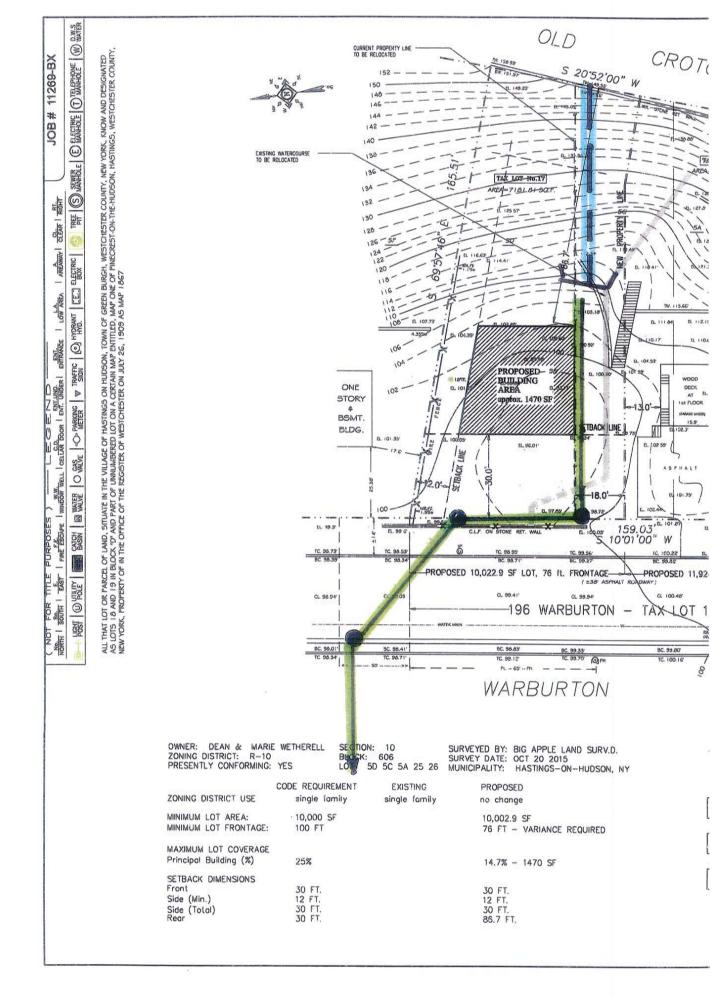
APPENDIX B

SITE LOCATIONS MAP

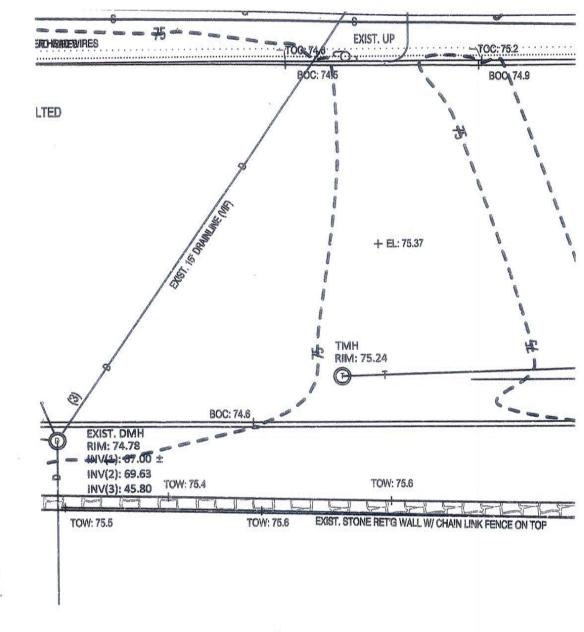
TAX MAP SHOWING THE DRAINAGE AREAS

LOT 1 WATERWAY AND DRAINAGE SYSTEM









Slopes Table — Total Site And Disturbed Area						
Number	Minimum Slope	Maximum Slope	Area (sq. ft.)	Percent	Disturbed area (sq. ft.)	Percent Disturbed
1	0	15	2,148	21.30%	1,644	16.30% 🎻
-2	15	25	1,913	19.00%	1,278	12.67%
	· 25	100	6,028	59.70%	960	9.52% 🎤
	,	6.	**	100%	3,882	
		TOTAL LOT	10,089			





APPENDIX C

STORMWATER FLOWS RATIONAL HYDROGRAPHS 1-4

Hydraflow Hydrographs by Intelisolve v9.1

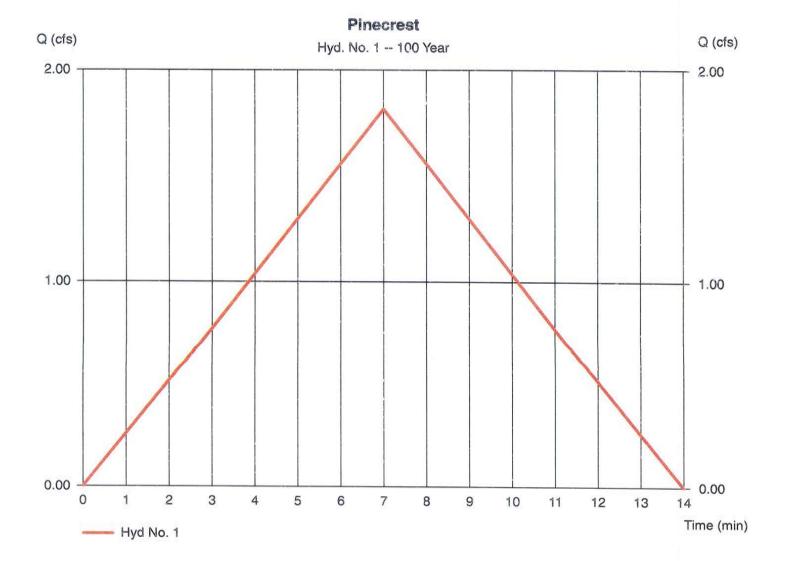
Friday, Apr 21, 2017

Hyd. No. 1

Pinecrest

Hydrograph type = Rational
Storm frequency = 100 yrs
Time interval = 1 min
Drainage area = 0.230 ac
Intensity = 8.779 in/hr
IDF Curve = My_Area.IDF

Peak discharge = 1.817 cfs
Time to peak = 0.12 hrs
Hyd. volume = 763 cuft
Runoff coeff. = 0.9
Tc by User = 7.00 min
Asc/Rec limb fact = 1/1



Hydraflow Hydrographs by Intelisolve v9.1

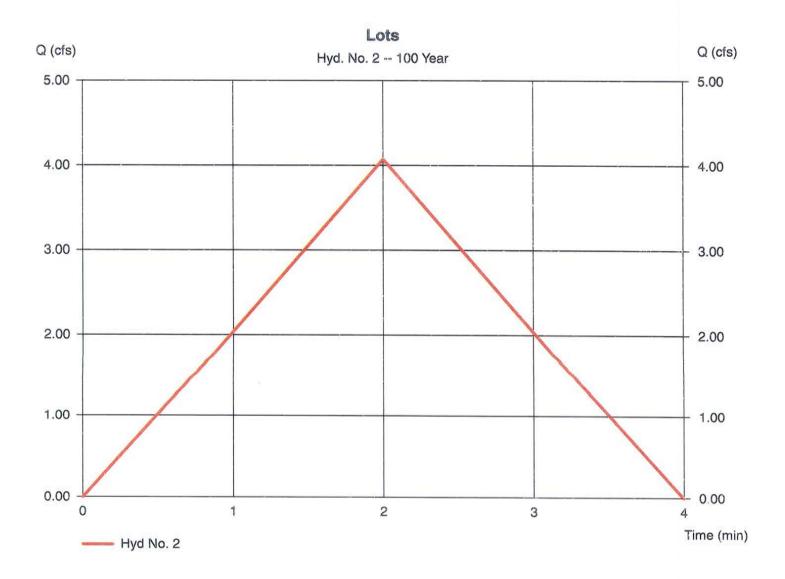
Friday, Apr 21, 2017

Hyd. No. 2

Lots

Hydrograph type = Rational
Storm frequency = 100 yrs
Time interval = 1 min
Drainage area = 0.610 ac
Intensity = 11.133 in/hr
IDF Curve = My_Area.IDF

Peak discharge = 4.075 cfs
Time to peak = 0.03 hrs
Hyd. volume = 489 cuft
Runoff coeff. = 0.6
Tc by User = 2.00 min
Asc/Rec limb fact = 1/1



Hydraflow Hydrographs by Intelisolve v9.1

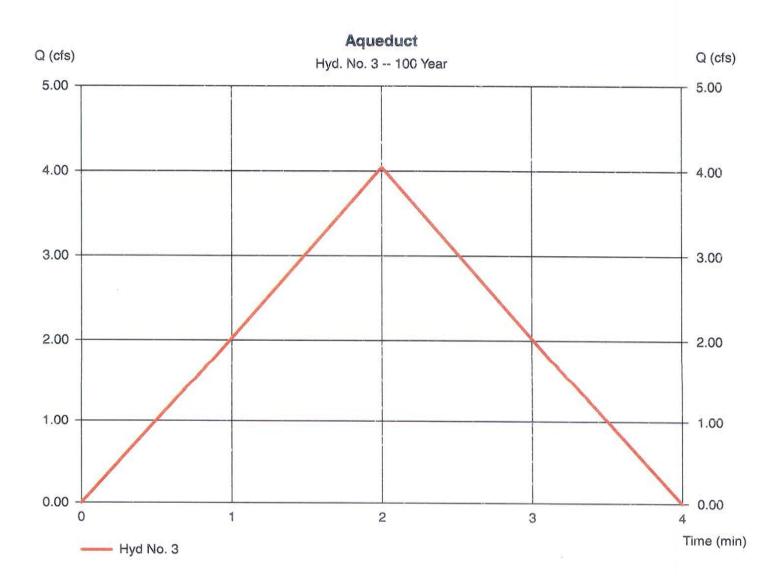
Friday, Apr 21, 2017

Hyd. No. 3

Aqueduct

Hydrograph type = Rational
Storm frequency = 100 yrs
Time interval = 1 min
Drainage area = 0.455 ac
Intensity = 11.133 in/hr
IDF Curve = My_Area.IDF

Peak discharge = 4.048 cfs
Time to peak = 0.03 hrs
Hyd. volume = 486 cuft
Runoff coeff. = 0.8
Tc by User = 2.00 min
Asc/Rec limb fact = 1/1



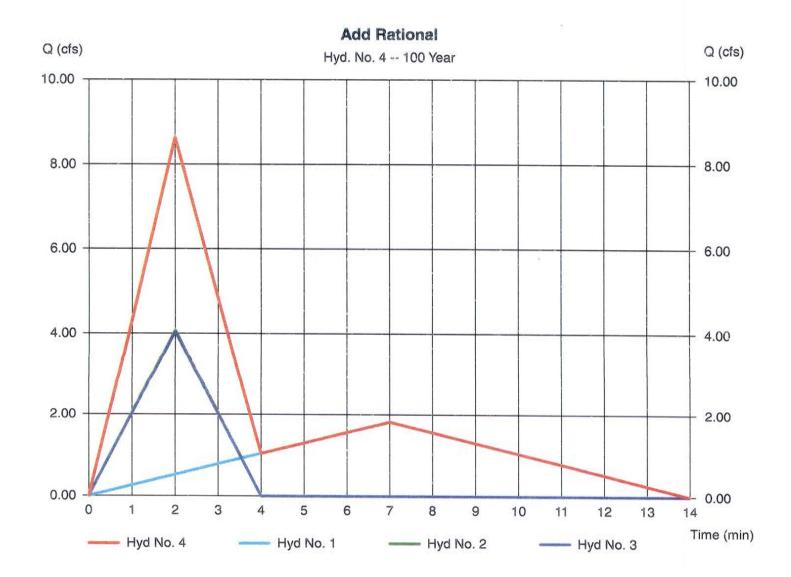
Hydraflow Hydrographs by Intelisolve v9.1

Friday, Apr 21, 2017

Hyd. No. 4

Add Rational

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min Inflow hyds. = 1, 2, 3 Peak discharge = 8.642 cfs Time to peak = 0.03 hrs Hyd. volume = 1,738 cuft Contrib. drain. area= 1.295 ac



APPENDIX D

STORMWATER FLOWS SCS TYPE III HYDROGRAPHS 5

Hydraflow Hydrographs by Intelisolve v9.1

Friday, Apr 21, 2017

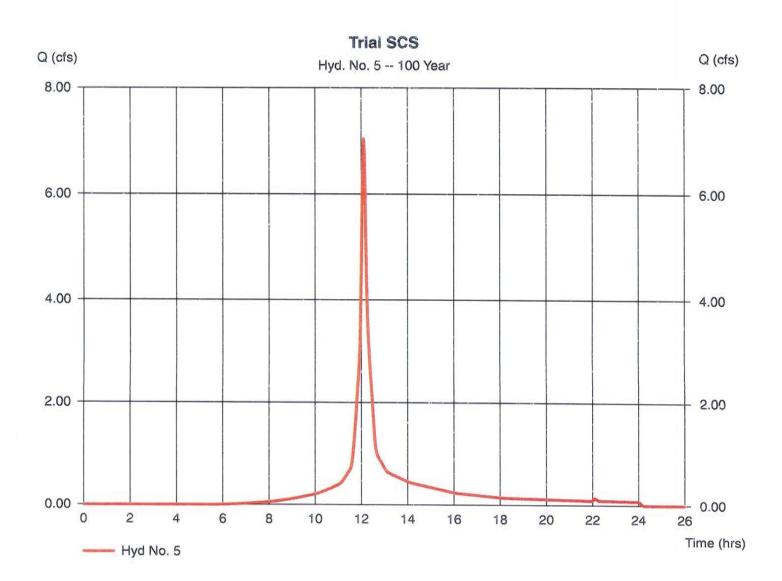
Hyd. No. 5

Trial SCS

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 2 minDrainage area = 1.300 acBasin Slope = 10.0 % Tc method = LAG Total precip. = 7.50 inStorm duration = 24 hrs

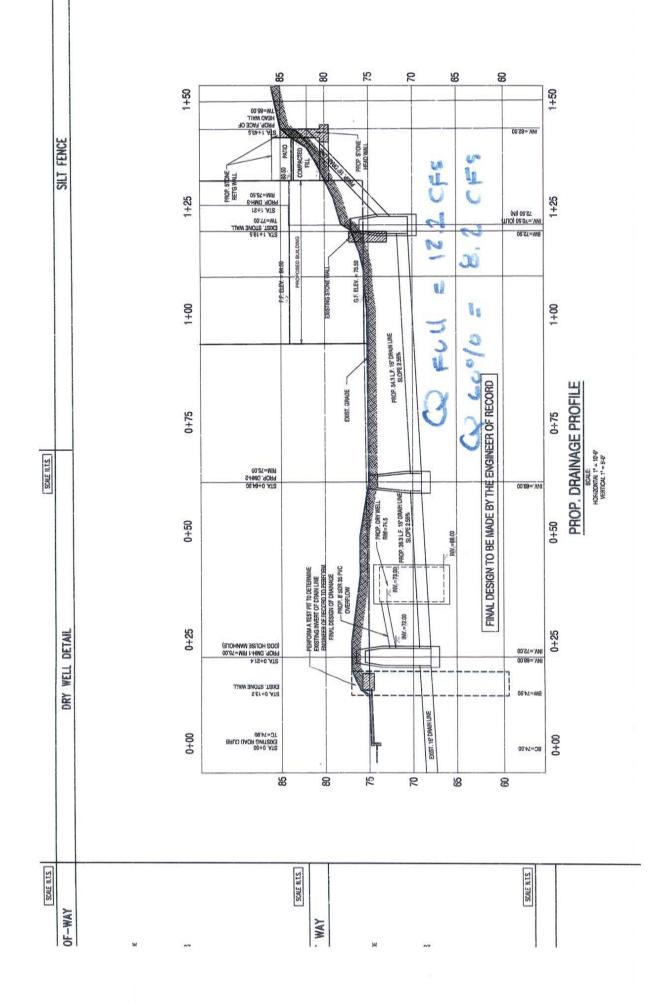
Peak discharge = 7.043 cfs
Time to peak = 12.10 hrs
Hyd. volume = 24,340 cuft

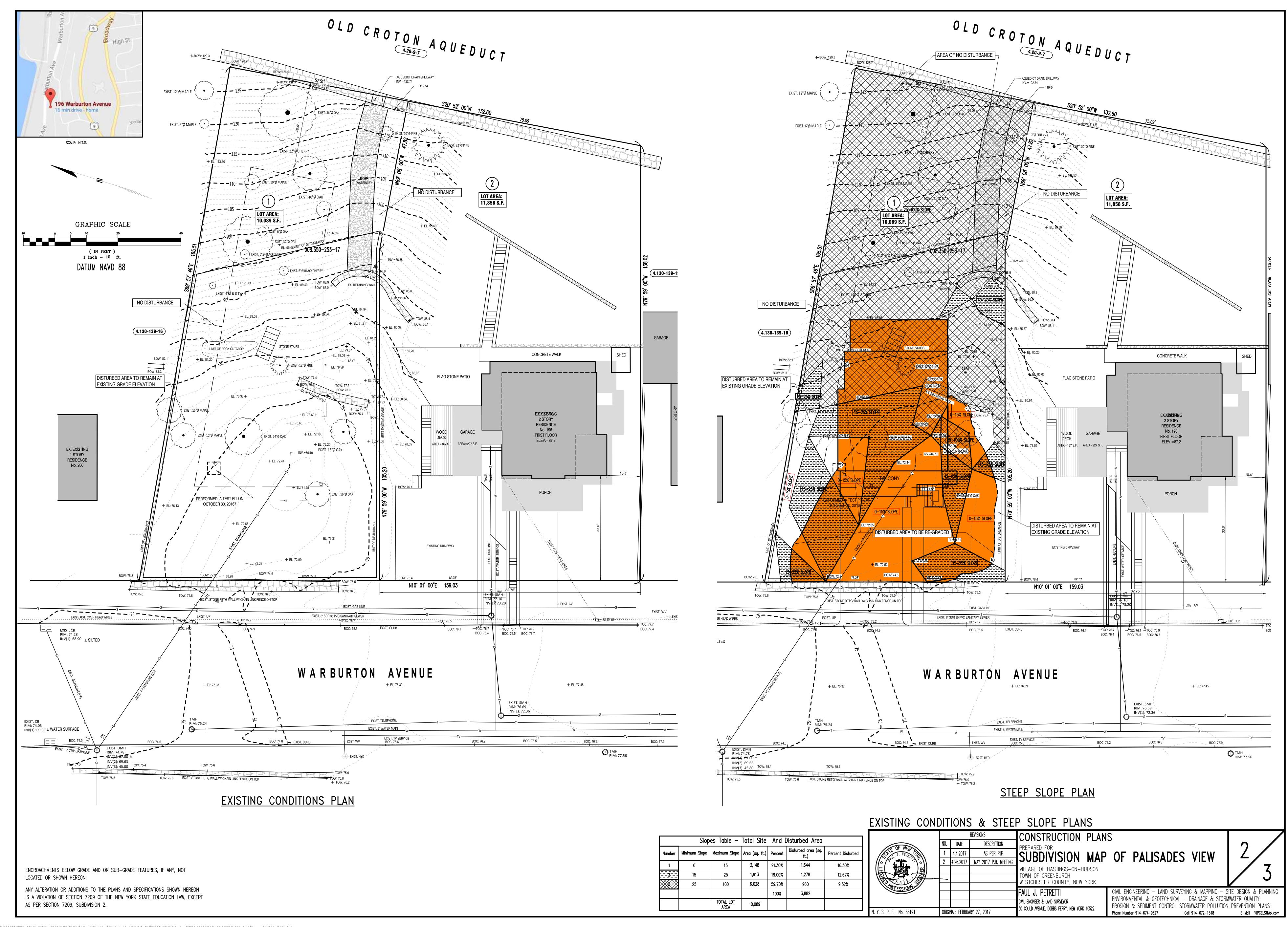
Curve number = 80
Hydraulic length = 600 ft
Time of conc. (Tc) = 6.69 min
Distribution = Type III
Shape factor = 484

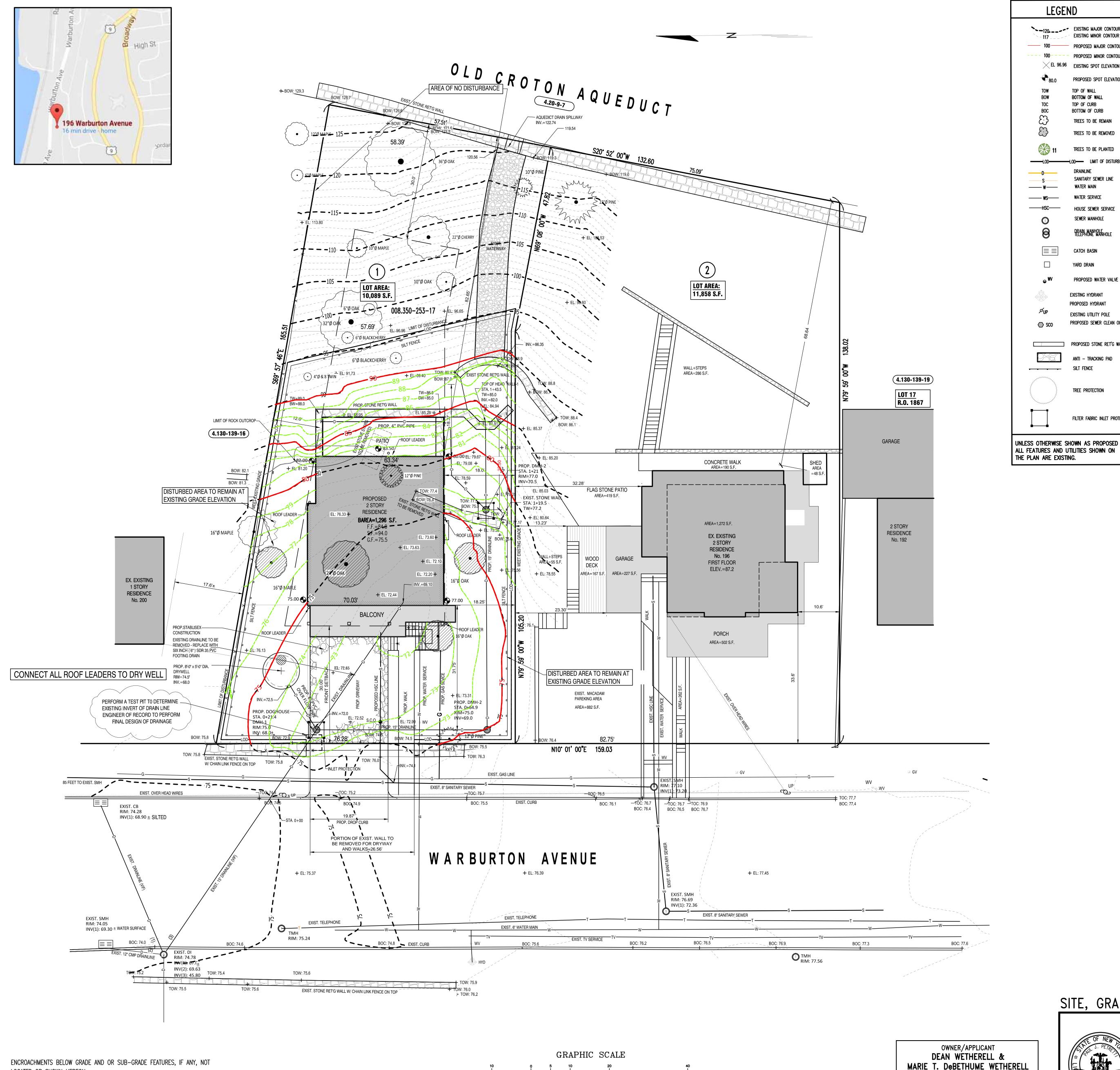


APPENDIX E

SITE DRAINAGE DESIGN







(IN FEET)

1 inch = 10 ft.

DATUM NAVD 88

EXISTING MINOR CONTOUR - PROPOSED MAJOR CONTOUR PROPOSED MINOR CONTOUR EL 96.96 EXISTING SPOT ELEVATION PROPOSED SPOT ELEVATION TOP OF WALL BOTTOM OF WALL TOP OF CURB BOTTOM OF CURB TREES TO BE REMAIN TREES TO BE REMOVED TREES TO BE PLANTED SANITARY SEWER LINE WATER MAIN WATER SERVICE HOUSE SEWER SERVICE SEWER MANHOLE CATCH BASIN YARD DRAIN PROPOSED WATER VALVE EXISTING HYDRANT PROPOSED HYDRANT EXISTING UTILITY POLE PROPOSED SEWER CLEAN OUT PROPOSED STONE RET'G WALL ANTI - TRACKING PAD TREE PROTECTION

FILTER FABRIC INLET PROTECTION

VILLAGE OF HASTINGS-ON-HUDSON GENERAL NOTES

1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION AND PRESERVATION OF ALL PUBLIC AND PRIVATE UNDERGROUND AND SURFACE UTILITIES AND STRUCTURES AT OR ADJACENT TO THE SITE OF THE CONSTRUCTION INSOFAR AS THEY MAY BE ENDANGERED BY HIS OPERATIONS. THIS SHALL HOLD TRUE WHETHER OR NOT THEY ARE SHOWN ON THE PROPOSED PLAN. IF THEY ARE SHOWN ON THE PROPOSED PLAN THEIR LOCATIONS ARE NOT GUARANTEED EVEN THOUGH THE INFORMATION WAS OBTAINED FROM THE BEST AVAILABLE SOURCES, AND IN ANY EVENT, OTHER UTILITIES NOT SHOWN ON THESE PLANS MAY BE ENCOUNTERED IN THE FIELD. THE CONTRACTOR SHALL, AT HIS OWN EXPENSE, REPAIR OR REPLACE ANY STRUCTURES OR UTILITIES THAT HE DAMAGES AND SHALL CONSTANTLY PROCEED WITH CAUTION TO PREVENT UNDUE INTERRUPTIONS TO UTILITY SERVICE.

- 2. THE CONTRACTOR SHALL VERIFY THE LOCATION OF EXISTING UTILITIES WHETHER UNDERGROUND OR OVERHEAD AND SHALL MAINTAIN IN PROPER WORKING AND OPERATING CONDITION ALL UTILITIES. IF TEMPORARY UTILITIES CONNECTIONS SERVICES ARE REQUIRED, THE CONTRACTOR SHALL SEE TO IT THAT THEY ARE PROVIDED, AND IT SHALL BE HIS RESPONSIBILITY TO MAINTAIN SUCH TEMPORARY FACILITIES FOR THE DURATION OF THE PROJECT.
- 3. THE CONTRACTOR SHALL CONTACT ALL OF THE APPROPRIATE PARTIES WITH JURISDICTION OVER THE UTILITIES ENTERING ON OR NEAR THE PROJECT AREA PRIOR TO THE INITIATION OF CONSTRUCTION ACTIVITIES,
- 4. THE CONTRACTOR'S ATTENTION IS DIRECTED TO THE STATE OF NEW YORK DEPARTMENT OF LABOR, BOARD OF STANDARDS AND APPEALS INDUSTRIAL CODE RULE 53," CONSTRUCTION, EXCAVATION AND DEMOLITION OPERATIONS AT OR NEAR UNDERGROUND FACILITIES" EFFECTIVE APRIL 1, 1975. THE CONTRACTOR WILL BE REQUIRED TO COMPLY WITH ALL APPLICABLE REQUIREMENT OF INDUSTRIAL CODE RULE 53. INFORMATION IS AVAILABLE AT 1-800-962-7962.
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED BY THE VILLAGE OF HASTING ON HUDSON, THE NEW YORK STATE DEPARTMENT OF TRANSPORTATION (NYSDOT) OR ANY OTHER AGENCY WITH JURISDICTION OVER THE PROJECT.
- 6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAKING ALL ARRANGEMENTS FOR ALL NECESSARY INSPECTIONS REQUIRED BY THE VILLAGE OF HASTING ON HUDSON OR ANY OTHER AGENCY WITH JURISDICTION OVER THIS PROJECT.
- 7. THE CONTRACTOR IS RESPONSIBLE FOR THE RESTORATION OR REPLACEMENT OF ALL (OFF-SITE) NATURAL AND MAN MADE FEATURES DISTURBED BY (HIS) CONSTRUCTION ACTIVITIES WHETHER SHOWN ON THE PROPOSED PLAN OR ENCOUNTERED IN THE FIELD. IT IS THE INTENTION OF THE PROPOSED PLAN TO SHOW ONLY MAJOR FEATURES TO BE PROTECTED, RESTORED OR REPLACED: HOWEVER, OTHER PHYSICAL FEATURES MAY BE ENCOUNTERED WHICH WILL REQUIRE RESTORATION OR REPLACEMENT WHETHER OR NOT THEY ARE SHOWN ON THE PLAN.
- 8. THE CONTRACTOR SHALL VERIFY ALL THE FIELD CONDITIONS AND DIMENSIONS AND SHALL BE RESPONSIBLE FOR FIELD FITTING AND QUANTITY OF WORK. NO ALLOWANCES SHALL BE MADE ON THE BEHALF OF THE CONTRACTOR FOR ANY ERROR OR NEGLECT ON HIS PART.
- 9. I HEREBY ACKNOWLEDGE THAT SUBSURFACE INVESTIGATIONS AND BORINGS HAVE NOT BEEN PERFORMED. SUBSURFACE INFORMATION IS NOT AVAILABLE FOR THIS PROJECT. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL SUBSURFACE CONDITIONS AND SATISFYING ALL CONTRACTUAL CONDITIONS WITH RESPECT TO UNFORESEEN CONDITIONS PRIOR TO ENTERING INTO AGREEMENT TO PERFORM THE WORK SHOWN

STEEP SLOPE CERTIFICATION

A PLAN SUBMITTED UNDER THE SEAL OF A LICENSED PROFESSIONAL ENGINEER SHOWING AND CERTIFYING THE FOLLOWING:

ALL EXISTING AND PROPOSED NATURAL AND ARTIFICIAL DRAINAGE COURSES AND OTHER FEATURES FOR THE CONTROL OF DRAINAGE, EROSION AND WATER.

THE PLANS SHOW ALL NATURAL DRAINAGE PATHS AND FEATURES.

THE CALCULATED VOLUME OF WATER RUNOFF FROM THE SLOPE(S) AND FROM THE LOT IN QUESTION, AS UNIMPROVED.

<u>VOLUME OF RUNOFF FROM THE SLOPES = AREA X RUNOFF COEFFICIENT X</u> 6 INCHES OF RAINFALL = AREA X 0.60 X 6"/12" = 2,572 S.F. X 0.60 X 6"/12" = 771.6 CUBIC FEET.

THE CALCULATED VOLUME OF WATER RUNOFF FROM THE SLOPE(S) AND FROM THE LOT IN QUESTION, AS IMPROVED.

<u>VOLUME OF RUNOFF FROM THE SLOPES = AREA X RUNOFF COEFFICIENT X</u> 6 INCHES OF RAINFALL = AREA X 0.60 X 6"/12" = 2.572 S.F. X 0.90 X 6"/12" = 1.157.4 CUBIC FEET.

THE EXISTENCE, LOCATION AND CAPACITY OF ALL NATURAL AND ARTIFICIAL DRAINAGE COURSES AND FACILITIES WITHIN 500 FEET OF THE LOT WHICH ARE OR WILL BE USED TO CARRY OR CONTAIN THE WATER

RUNOFF FROM THE SLOPE(S) AND THE LOT. THE PLANS SHOW ALL NATURAL AND ARTIFICAL DRAINAGE COURSES AND FACILITES.

A STATEMENT MADE UNDER THE SEAL OF A LICENSED PROFESSIONAL ENGINEER CERTIFYING THAT:

THE PROPOSED ACTIVITY WILL DISTURB THE STEEP SLOPE AREA TO THE MINIMUM EXTENT POSSIBLE; AND

THE PROPOSED ACTION, THE CONSTRUCTION OF A HOUSE AND RELATED FEATURES WILL IMPACT THE STEEP SLOPES TO THE MINIMUM EXTENT POSSIBLE, THE STEEPEST AND WOODED PORTION OF THE LOT WITH THE DRAINAGE WAY THAT CONVEYS RUNOFF FROM THE OLD CROTON AQUEDUCT WILL NOT BE DISTURBED.

THE PROPOSED MITIGATION MEASURE WILL PREVENT, TO THE MAXIMUM EXTENT PRACTICAL, THE ADVERSE EFFECT OF ANY DISTURBANCE OF THE STEEP SLOPE AREA ON THE ENVIRONMENT AND ANY NEIGHBORING

RUNOFF FROM THE STEEP SLOPES, ESPECIALLY THAT FROM THE OLD CROTON AQUEDUCT DRAIN WILL BE CONVEYED TO THE EXISTING DRAINAGE WORKS BY WAY OF A NEW DRAIN LINE AND THE LAND DISTURBANCE WILL NOT HAVE ANT ADVERSE IMPACT ON THE STEEP SLOPES, THE ENVIRONMENT AND ANY NEIGHBORING PROPERTIES. RUNOFF FROM THE DISTURBED STEEP SLOPES, THE IMPROVED AREA WITH THE HOUSE AND DRIVEWAY AND RELATED IMPERVIOUS AREA WILL BE CONNECTED TO A DRYWELL WITH AN OVERFLOW TO THE PROPOSED

APPROVAL NOTES

"THE VILLAGE ENGINEER AND BUILDING INSPECTOR MAY REQUIRE ADDITIONAL EROSION CONTROL MEASURES IF DEEMED APPROPRIATE TO MITIGATE UNFORESEEN SILTATION AND EROSION OF DISTURBED SOILS." "AS BUILT" DRAWINGS OF THE SITE IMPROVEMENTS SHALL BE SUBMITTED TO THE VILLAGE ENGINEER AND BUILDING INSPECTOR FOR REVIEW PRIOR TO OBTAINING CERTIFICATION OF OCCUPANCY" "NO DEMOLITION MATERIAL TO BE BURIED ON SITE".

CUT AND FILL

CUT = 115 CUBIC YARDS

NET EXPORT = 103 CUBIC YARDS

FILL = 12 CUBIC YARDS

R-10 Zo	ning Table Lot-	-1
	Required	Lot 1
AREA (sq. ft.)	10,000	10,088
Width	100	65.76 *
Front yard (ft.)	30	30.17
Side Yard (ft.)	12	12.17
2 Side yards (ft.)	30	30.00
Rear Yard (ft.)	30	72.81
Building Coverage (%)	25%	18.45%
Development Coverage (%)	35%	25.50%
Height of Building (ft.)	35	>35

^{*} VARIANCE REQUIRED

R-10 Zoning Table Lot-2				
	Required	Lot 2		
AREA (sq. ft.)	10,000	11,858		
Width	100	81.60 *		
Front yard (ft.)	30	33.6 **		
Side Yard (ft.)	12	10.6 **		
2 Side yards (ft.)	30	23.8 **		
Rear Yard (ft.)	30	82.7 **		
Building Coverage (%)	25%	14.21%		
Development Coverage (%)	35%	36.35% *		
Height of Building (ft.)	35	<35		

VARIANCE REQUIRED

SITE, GRADING AND UTILITY

ORIGINAL: FEBRUARY 10, 2017

N. Y. S. P. E. No. 55191

196 WARBURTON AVENUE

HASTINGS -ON-HUDSON

NEW YORK 10706

PHONE No. 1-914-584-0742

CONSTRUCTION PLANS NO. DATE DESCRIPTION -SUBDIVISION MAP OF PALISADES VIEW MAY 2017 P.B. MEETING VILLAGE OF HASTINGS-ON-HUDSON TOWN OF GREENBURGH WESTCHESTER COUNTY, NEW YORK

PAUL J. PETRETTI

30 GOULD AVENUE, DOBBS FERRY, NEW YORK 10522.

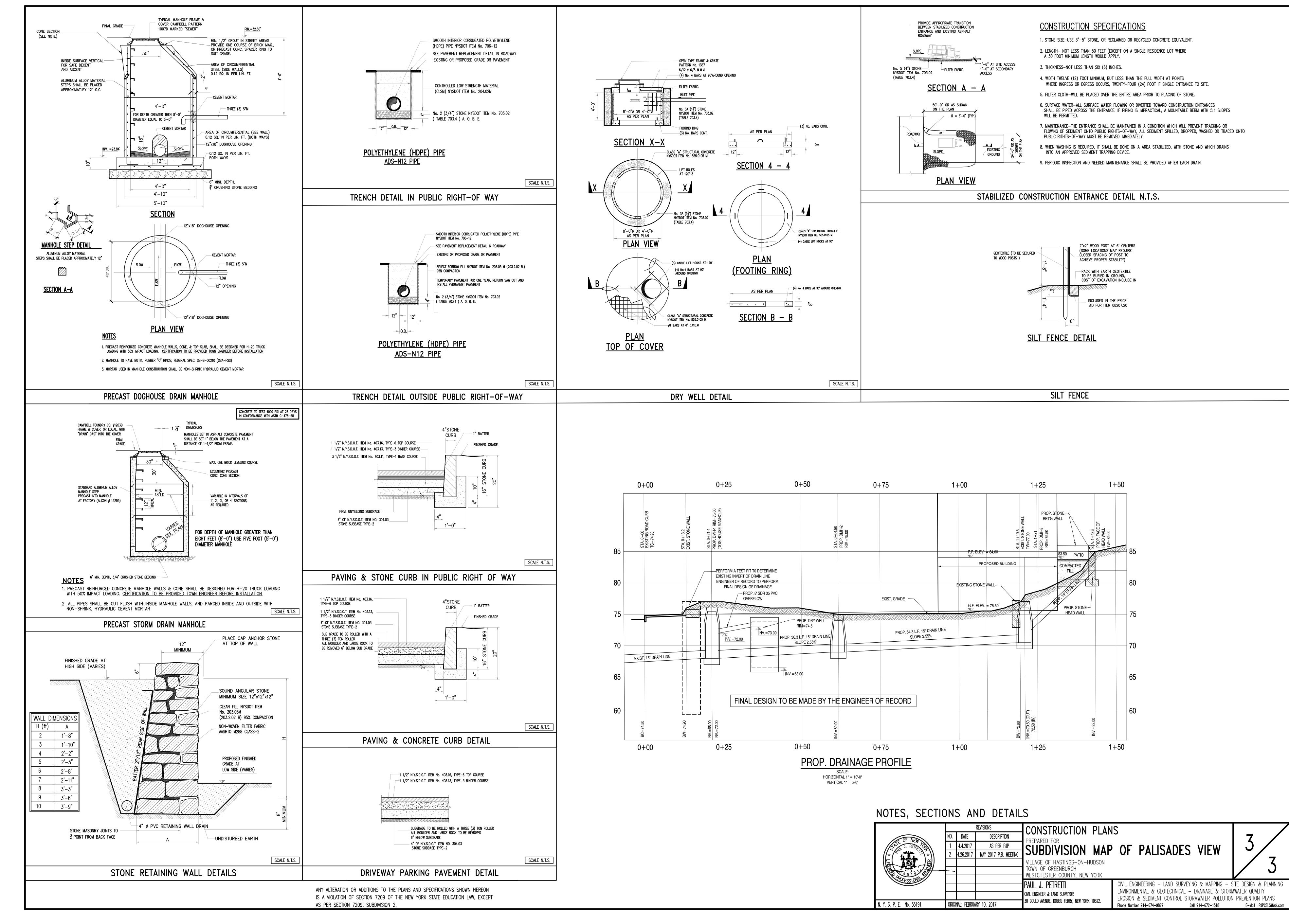
CIVIL ENGINEER & LAND SURVEYOR

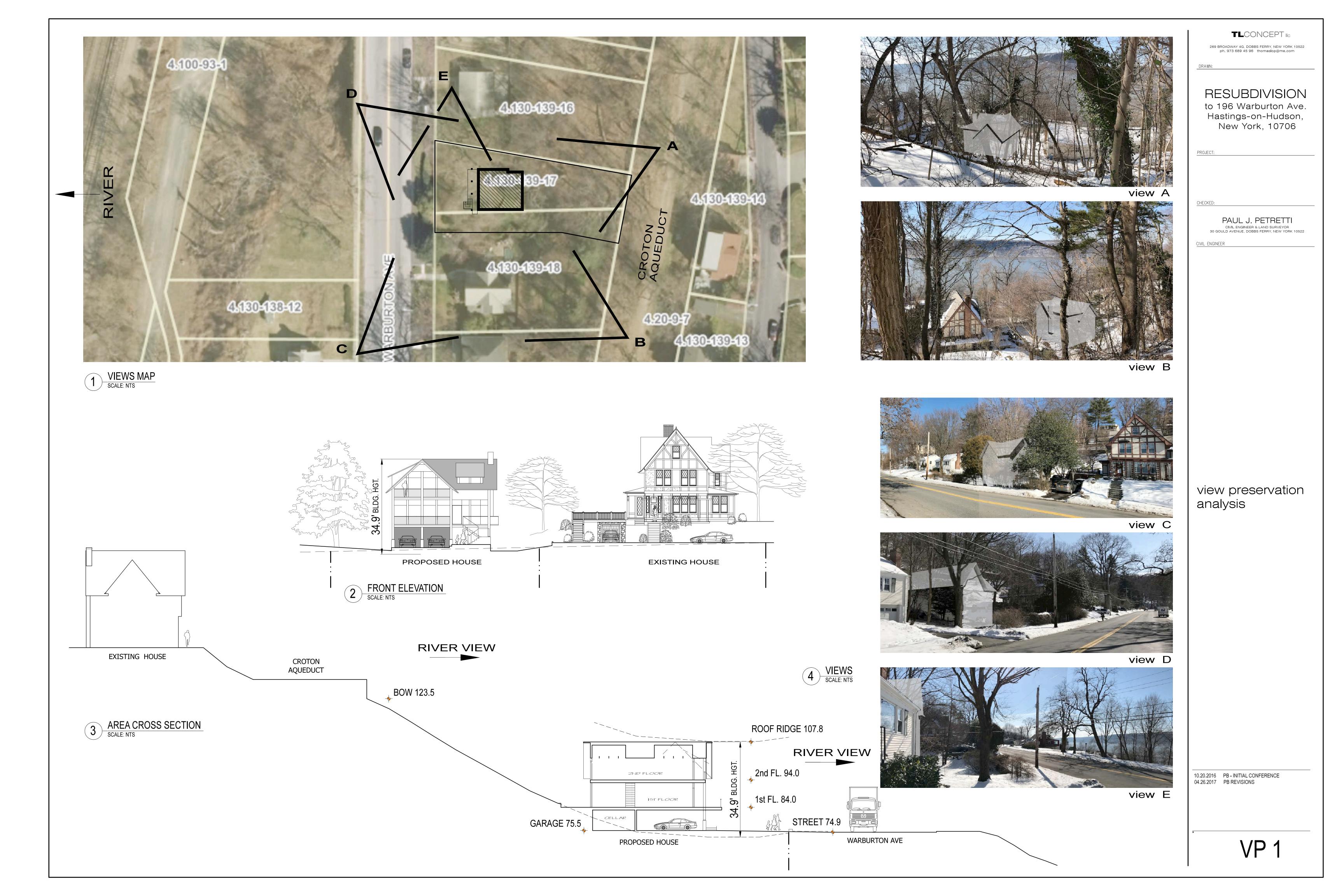
CIVIL ENGINEERING - LAND SURVEYING & MAPPING - SITE DESIGN & PLANNING ENVIRONMENTAL & GEOTECHNICAL - DRAINAGE & STORMWATER QUALITY EROSION & SEDIMENT CONTROL STORMWATER POLLUTION PREVENTION PLANS Phone Number 914-674-9827 Cell 914-672-1518 E-Mail PJPCELS@Aol.com

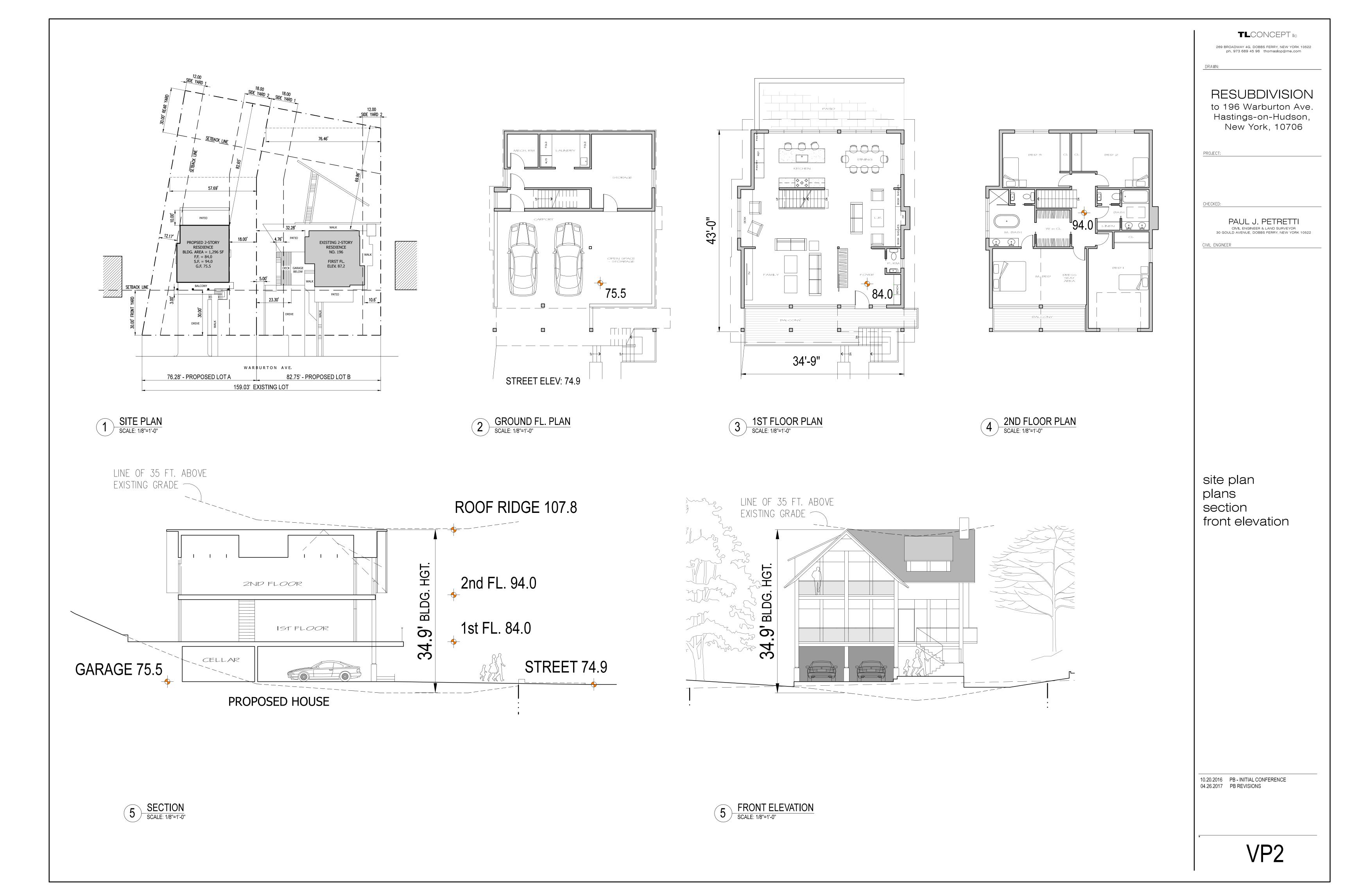
ANY ALTERATION OR ADDITIONS TO THE PLANS AND SPECIFICATIONS SHOWN HEREON IS A VIOLATION OF SECTION 7209 OF THE NEW YORK STATE EDUCATION LAW, EXCEPT AS PER SECTION 7209, SUBDIVISION 2.

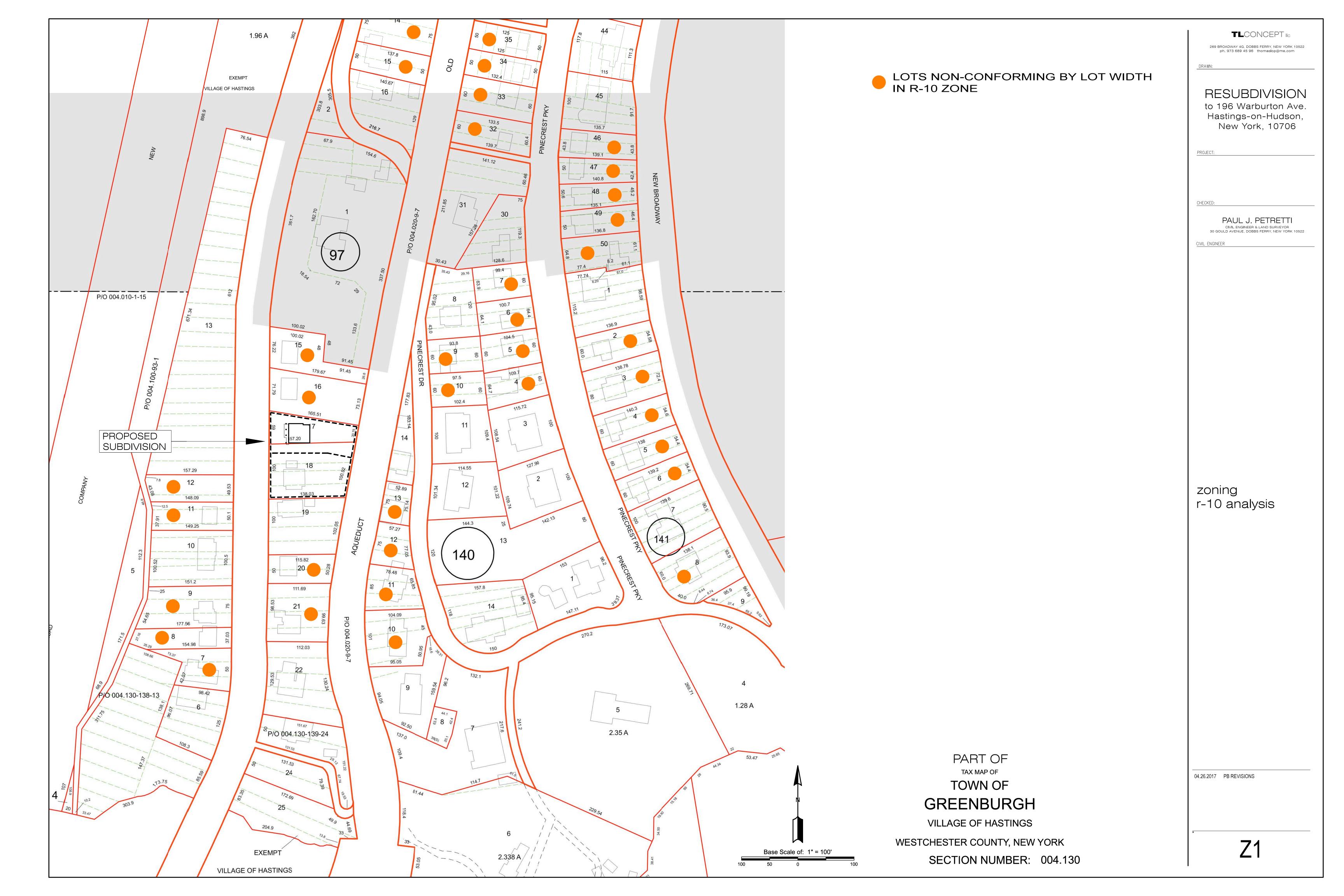
LOCATED OR SHOWN HEREON.

Z:\CIVIL 3D PROJECTS\NAD83-NAVD88\VILLAGE OF HASTINGS\004.130\Block 139\Lot 16 - 196 Warburton\dwg\20170207 - Site Plan.DWG, SHEET 1, 4/27/2017 9:06:44 AM, DWG To PDF.pc3, ARCH full bleed E1 (30.00 x 42.00 Inche









30.00' REAR YARD 12.00 SIDE YARD 2 76.46 57.69 10.00 PATIO WALK 12.17' 4.76' PATIO 18.00' **EXISTING 2-STORY** PROPSED 2-STORY RESDIENCE RESDIENCE NO. 196 BLDG. AREA = 1,296 SF | WALK F.F. = 84.0S.F. = 94.0FIRST FL. ELEV. 87.2 G.F. 75.5 DECK GARAGE BELOW WALK BALCONY SETBACK LINE PATIO 30.00' FRONT YARD 10.6 23.30' DRIVE DRIVE WARBURTON AVE. 76.28' - PROPOSED LOT A 82.75' - PROPOSED LOT B 159.03' EXISTING LOT

R-10 ZONING TABLE LOT - 1

	REQUIRED	PROPOSED
MIN. LOT AREA (sq. ft.)	10,000	10,088
MIN. WIDTH (ft.)	100	65.76
MIN. FRONT YARD (ft.)	30	30.17
MIN. SIDE YARD (ft.)	12	12.17
MIN. 2 SIDE YARDS (ft.)	30	30.00
MIN. REAR YARD (ft.)	30	82.65
MAX. BUILDING COVERAGE (%)	25	18.45
MAX. DEVELOPMENT COVERAGE (%)	35	25.50
MAX. HEIGHT OF BUILDING (FT.)	35	34.9

R-10 ZONING TABLE LOT - 2

	REQUIRED	PROPOSED	
MIN. LOT AREA (sq. ft.)	10,000	11,858	
MIN. WIDTH (ft.)	100	81.60	*
MIN. FRONT YARD (ft.)	30	33.60	*
MIN. SIDE YARD (ft.)	12	10.6	*
MIN. 2 SIDE YARDS (ft.)	30	33.90	
MIN. REAR YARD (ft.)	30	82.70	*
MAX. BUILDING COVERAGE (%)	25	14.21	
MAX. DEVELOPMENT COVERAGE (%)	35	36.35	*
MAX. HEIGHT OF BUILDING (FT.)	35	< 35	

note: 6 ft. setback encroachment is allowed for decks

* VARIANCE REQUIRED** EXISTING

TLCONCEPT IIC

269 BROADWAY 4G, DOBBS FERRY, NEW YORK 10522
ph. 973 689 45 96 thomaslop@me.com

RESUBDIVISION to 196 Warburton Ave.

Hastings-on-Hudson, New York, 10706

PAUL J. PETRETTI

CIVIL ENGINEER & LAND SURVEYOR
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zoning calculations

04.26.2017 PB REVISIONS

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